

February 8, 2018

File No.: 161-17798-00

Thomas Burr, Vice President  
Suite 200, 12420-104 Avenue NW  
Edmonton, AB  
T5N 3Z9

**RE: SITE SERVICING DESIGN BRIEF FOR 99<sup>TH</sup> STREET MIXED USE DEVELOPMENT  
REVISION #4 FEBRUARY 8, 2018 (SANITARY & STORM SERVICING)**

Dear Thomas;

## **INTRODUCTION**

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WSP Canada Inc. on behalf of ONE Properties, has completed a servicing design brief on the above noted lands for the proposed rezoning to a mixed use development. This proposed development consists of 2 tower structures over a 3 level underground parkade. The west tower is 18 storeys of residential units. The main and second floors contain 5 Town House style units. The east tower is 15 storeys and is mixed use. The main and second floors comprise a commercial podium and the remaining tower is residential. In total, there are 242 proposed residential units between the two towers. The site is located at the NW corner of 99 Street and 89 Avenue and is approximately 0.35 hectares in size. The location plan as seen in the accompanied "Figure A" shows the proposed locations of the development and the rezoning of 8 lots: Lots 1-5 and Lots 26-28, Block 122, Plan RN27.

The uses of the proposed commercial space for this development are currently unknown. For the purpose of this brief, it is assumed that all the retail space will be restaurants. This is a conservative assumption that considers the highest probable sanitary flows. However, it may be likely that some of the area may be retail shopping or grocery which produces less wastewater. The sanitary flow generation calculations will be adjusted to reflect the actual proposed usage at the time of detailed design.

The preliminary alignment of the deep underground utilities, capacity restriction solutions, and service connection locations will be finalized during detailed design upon further discussions between WSP, EPCOR Water and Sewer Services (WASS) department, and the City of Edmonton (COE) Drainage department.

## **SANITARY AND STORM**

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### **Design Criteria**

The post development sanitary flows, calculated from current City of Edmonton sanitary sewage generation rates, shows that the site will produce approximately 10.49 L/s of sanitary flow. New sanitary sewers in the City are to be designed to carry the design flow at a flow depth of 80% of the sewer diameter. This results in a flow rate of approximately 86% of the sewers' full flow capacity. The available capacity based on this expected flow will be approximately  $10.59/0.86 = 12.31$  L/s as shown in the figure shown below. Jatinder

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Tiwana with the City of Edmonton Drainage Department has confirmed via meeting and confirmation email that since the existing sewer system capacity is already strained in certain areas, the sanitary flow rate without the 0.86 is sufficient for design for the existing system.

Location	From MH	To MH	Added Lot ha	Added Road ha	Total Lot ha	Total Area ha	Added Avg Flow l/s	Tot Avg Flow l/s	Peak Factor	PDW Flow l/s	Ill Allow l/s	Sag MH's ea	Tot Sag MH's ea	MH Inflow l/s	PWW Flow l/s	Req. Flow l/s	Pipe Size mm	Slope %	Length m	Pipe Cap. l/s	Full Vel. m/s	PDWF Vel. m/s	
	LOT	MH1	0.355	0.00	0.355	0.36	0.07	0.07	32.88	10.49	0.10	0	0	0.00	10.59	12.31	150	2.000	7.36	2154	1.22	1.21	
<b>Classification</b>						<b>2</b>	<b>Floor</b>			<b>Area ft2</b>	<b>Area m2</b>	<b>L/Day/m2</b>		<b>L/S</b>	<b>Pf calc</b>	<b>Max Pf</b>	<b>Pf</b>	<b>QPDW</b>					
Commercial			Retail CRU-CAFE (Restaurant)				NA			9776	908.220119			20	0.2102	20.17	25	20					4.241
			CRU-Grocer				NA						8										
			Garbage/Service				1			1846	171.4990118			8	0.0159	64.51	25	25					0.397
			Amenity Space (office)							5583	518.6776723			8	0.048	39.20	25	25					1.201
Residential:			<b>Units</b>	<b>Net Area ha</b>		<b>People/Unit</b>	<b>People/ha.</b>			<b>Population</b>	<b>G (L/Day/ Person)</b>			<b>L/s</b>	<b>Pf</b>								<b>QPDW</b>
			237			1.909029536				452.44				300	1.571	2.81							4.422
	TH-L1		5			3.32				16.6				300	0.0576	3.92							0.226
<b>Total QPDW</b>																						10.486	

The storm system for this development is required to be designed to accommodate the 1 in a 100 year return frequency rainfall event. Due to the capacity restrictions in the existing downstream system, the City of Edmonton Drainage department is requiring that this site specifically be restricted to a release rate of 11.27 L/s/ha, resulting in a flow of 4 L/s generated from the site. This has been confirmed via meeting and email by Jatinder Tiwana. With a combined runoff coefficient of 0.95 (determined from hardscaped and landscaped surface areas) and a development area of 0.355 ha, the onsite storage requirement for the 1 in 100 year storm event for a 24 hour drawdown period is 185.58 m<sup>3</sup>. The precise method of onsite storage and location of the service connection point will be established at the detailed design phase.

From MH	To MH	Added Ha	Total Ha	C	CA	Total CA	Initial Time	Time in Pipe	Total Time	Int. mm/hr	Design Flow L/s	Design Flow L/s/ha	Total Flow L/s	Length m	Pipe Size mm	Slope %	Cap. L/s	Cap. Ratio	Full Vel. m/s	Design Vel. m/s																
	LOT	0.35	0.35									35 L/s/ha																								
Building	1	0.00	0.35	0.89	0.00	0.00	8.00	0.30	8.30	76.68	0	12	15.6	150	1.00	16	0.78	0.869	0.000																	
<b>NOTES:</b>																																				
Rims, Inverts, and Depths are approximate only - Refer to detail design drawings.																																				
<table style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="text-align: left;">Land Use</th> <th style="text-align: left;">C (Runoff Coef)</th> <th style="text-align: left;">Area (ha C x A)</th> </tr> </thead> <tbody> <tr> <td>1) Landscaped Area</td> <td>0.1</td> <td>0.027 0.0027</td> </tr> <tr> <td>2) Hardscape Surface</td> <td>0.95</td> <td>0.326 0.3097</td> </tr> <tr> <td><b>Total</b></td> <td></td> <td><b>0.353 0.3124</b></td> </tr> <tr> <td colspan="2" style="text-align: right;"><b>Combined C =</b></td> <td><b>0.885</b></td> </tr> </tbody> </table>																						Land Use	C (Runoff Coef)	Area (ha C x A)	1) Landscaped Area	0.1	0.027 0.0027	2) Hardscape Surface	0.95	0.326 0.3097	<b>Total</b>		<b>0.353 0.3124</b>	<b>Combined C =</b>		<b>0.885</b>
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Calculated as per the COE Drainage design standards, the total of the flows generated from the storm and sanitary systems will yield a total flow value of 14.49 L/s.

## **Existing Conditions**

The site cadastral, servicing reports, and combined main-line capacities have been provided to WSP by the WASS department. WSP has also met with the COE Drainage System Assessment department to address the existing design capacities and potential constraints.

### **Sanitary**

The servicing information provided by WASS for building 1 & 2 (as shown on the attached “Figure B”) indicates that the existing sanitary service connects to the existing 200 mm combined sewer on 99<sup>th</sup> street. Pre-development flow calculations produced by these two combined areas is approximately 5.11 L/s, yielding a required available capacity of 5.99 L/s.

The servicing information provided for building 3 indicates that the sanitary service connects to the existing 200mm combined sewer on 89<sup>th</sup> avenue. The pre-development flow calculation produced is approximately 2.08 L/s yielding a required available capacity of 2.46 L/s.

### **Storm**

Pre-development flows have been generated to determine the overland flows entering the combined sewer system. A runoff coefficient of 0.95 was used for the existing sites. Based on existing topographical information it has been assumed that the site runoff is currently uncontrolled flow. Using the site area of 0.355 ha, and a runoff coefficient of 0.95, the total pre-developmental flow generated by the site is approximately 91L/s.

### **System Constraints** (See “Figure C” for reference)

- 1) The 200mm combined main on 99<sup>th</sup> street north of 89<sup>th</sup> avenue has a total design capacity of 35.34 L/s.
- 2) The 2 mains on 100<sup>th</sup> street, north of 89<sup>th</sup> avenue have a combined capacity of 360 L/s. However, the direction of flow is partially uncontrolled at the intersection of 100<sup>th</sup> street and 89<sup>th</sup> avenue between the west and northwest inverts. The flow is initially directed through the north invert, but the flow could possibly flow further west on 89<sup>th</sup> and that 200mm main west on 89<sup>th</sup> avenue has a total capacity of 13 L/s. It could back up and flow north though the north-west and eventually travel down 100<sup>th</sup> Ave within the two parallel combined mains.
- 3) The 200mm combined main on 89<sup>th</sup> avenue east of the 99<sup>th</sup> street has a total design capacity of 30 L/s. This main ultimately connects to the 1500mm Combined Sewer Overflow (CSO) trunk main at 98<sup>th</sup> street and 88<sup>th</sup> avenue. This 1500mm main continues east along 88<sup>th</sup> avenue.

- 4) There have been reported issues with downstream surcharging and flooding around the residential areas along 91<sup>st</sup> Ave. between 98<sup>th</sup> St. and 99<sup>th</sup> St. during high storm flow events.

From recent correspondence between the COE Drainage System Assessment department and WSP, the combined sewer mains listed above in the site constraints are assumed to have limited capacity to fulfill the preliminary demands of the proposed development, however no modeling of the system for this area is available.

#### **Proposed Developmental Flows & Service Connections** (See “Figure D” for reference)

Existing design capacities were received from and discussed with COE Drainage System Assessment department, though no existing flows were available to be provided or discussed. Based on the design information received, and additional requirements provided by the Drainage department, Drainage proposed that the development connect the sanitary and storm services storm separately from 89<sup>th</sup> avenue. The reasoning is that any improvements required will decrease in cost if completed on 89<sup>th</sup> Ave in comparison to installation on 99<sup>th</sup> St.; 89<sup>th</sup> St. will be easier for traffic control; and there is more downstream capacity for the flows along the twin combined sewers along 100<sup>th</sup> St. Also, there are current surcharging and flooding issues into the east properties from the 99<sup>th</sup> St. main along 91<sup>st</sup> Ave towards 98<sup>th</sup> St. Servicing to the 99<sup>th</sup> St. main as-is will magnify the issue, unless the main is replaced and upsized for a significant length north of the proposed development. This is a less feasible option.

The sanitary contribution to the existing combined sewer on 89<sup>th</sup> St. will increase from the pre-development flow of 2.08 L/s to the calculated post development flow of 10.48 L/s. Although this is an increase in the sanitary contribution, this is ultimately a reduction to the existing combined (sanitary and storm) sewer if the portion of pre-developed uncontrolled storm flow currently entering the system is considered. As per a meeting held with Jatinder, Donovan Pederson, and Mikaela Hanley on March 1<sup>st</sup>, 2017, COE Drainage is requiring additional sanitary storage. On-site storage is higher-risk to the developer, but the option can be further explored during preliminary and detailed design. The alternative option is that the combined sewer on 89<sup>th</sup> Ave be upsized for increased storage capacity. Drainage recommends upsizing the 200 mm clay tile combined sewer along 89<sup>th</sup> Ave from 99<sup>th</sup> St to 100<sup>th</sup> St with a 600 mm pipe. This is the shortest length of upsizing option in comparison to extending north on 99<sup>th</sup> St or going east on 89<sup>th</sup> Ave to 98<sup>th</sup> St.

If required, the actual direction and the system distribution routes of the current uncontrolled storm flow can be further addressed when additional topographical information is available. Further considerations for including Low Impact Development (LID) features can be also be examined to further reduce the coefficient and thus decreasing the required storage volume. The storm service connection to 89<sup>th</sup> avenue will maintain a release rate of 11.27 L/s/ha for the proposed development which corresponds to 4 L/s of flow generated with 185.58 m<sup>3</sup> allocated to onsite storage. This significantly reduces the current overall uncontrolled storm flow of approximately 91 L/s.

## SUMMARY

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This brief has investigated the direct options for servicing the proposed development with storm and sanitary. Utilizing the measures and preliminary assumptions as outlined above in this servicing brief, it appears that since the storm will now be controlled then the post-developmental flows will not increase the total contribution to the existing combined systems. The options provided in this report have been reviewed by the City of Edmonton and can be further assessed by the respective departments and WSP to finalize the approved servicing methods during detailed design.

Sincerely,

P. Bradley Clarke, P.Eng.  
Senior Project Manager  
Infrastructure

pbk

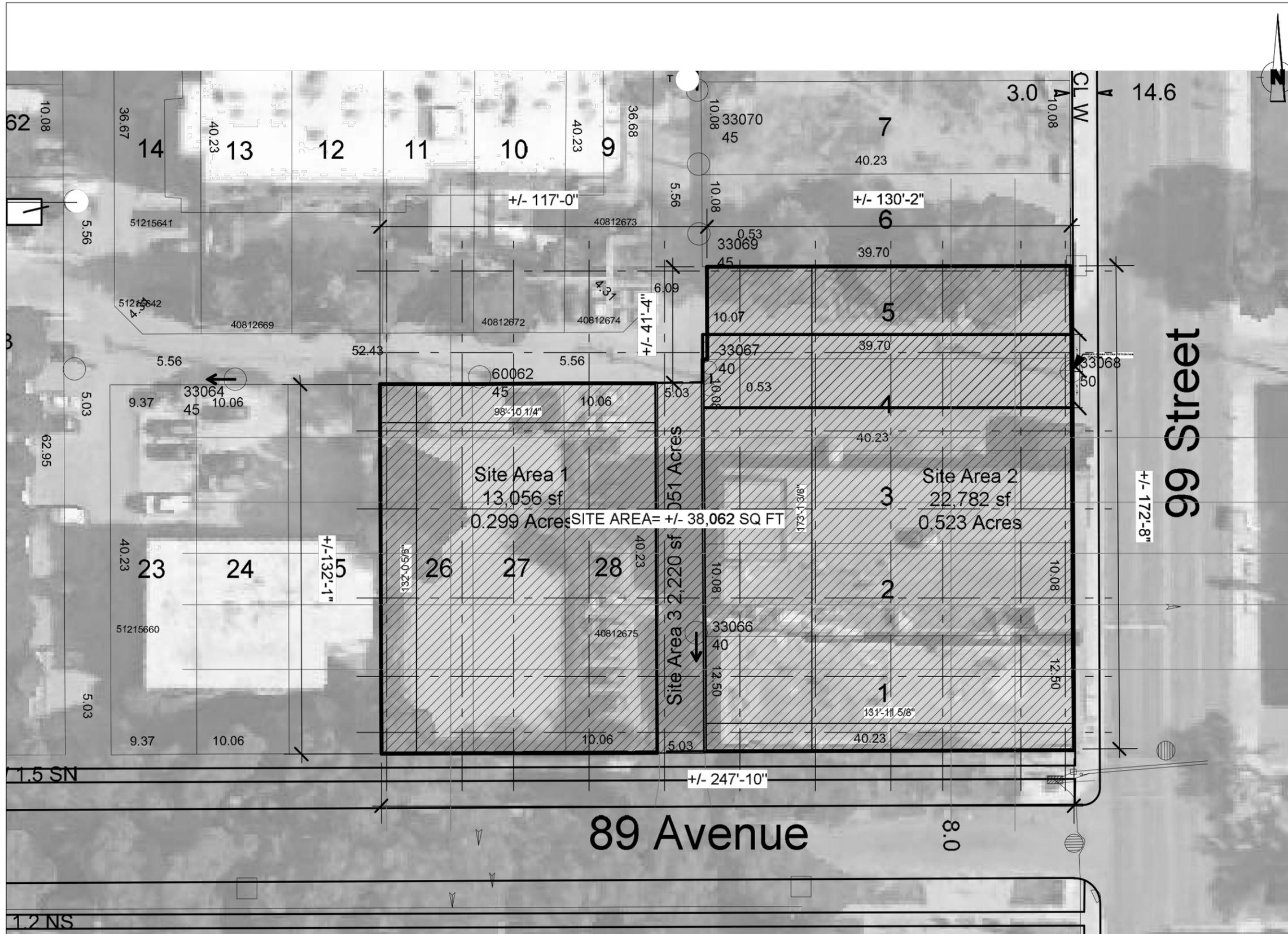
APEGA Permit to Practice P-7641

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Appendix A

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FIGURES



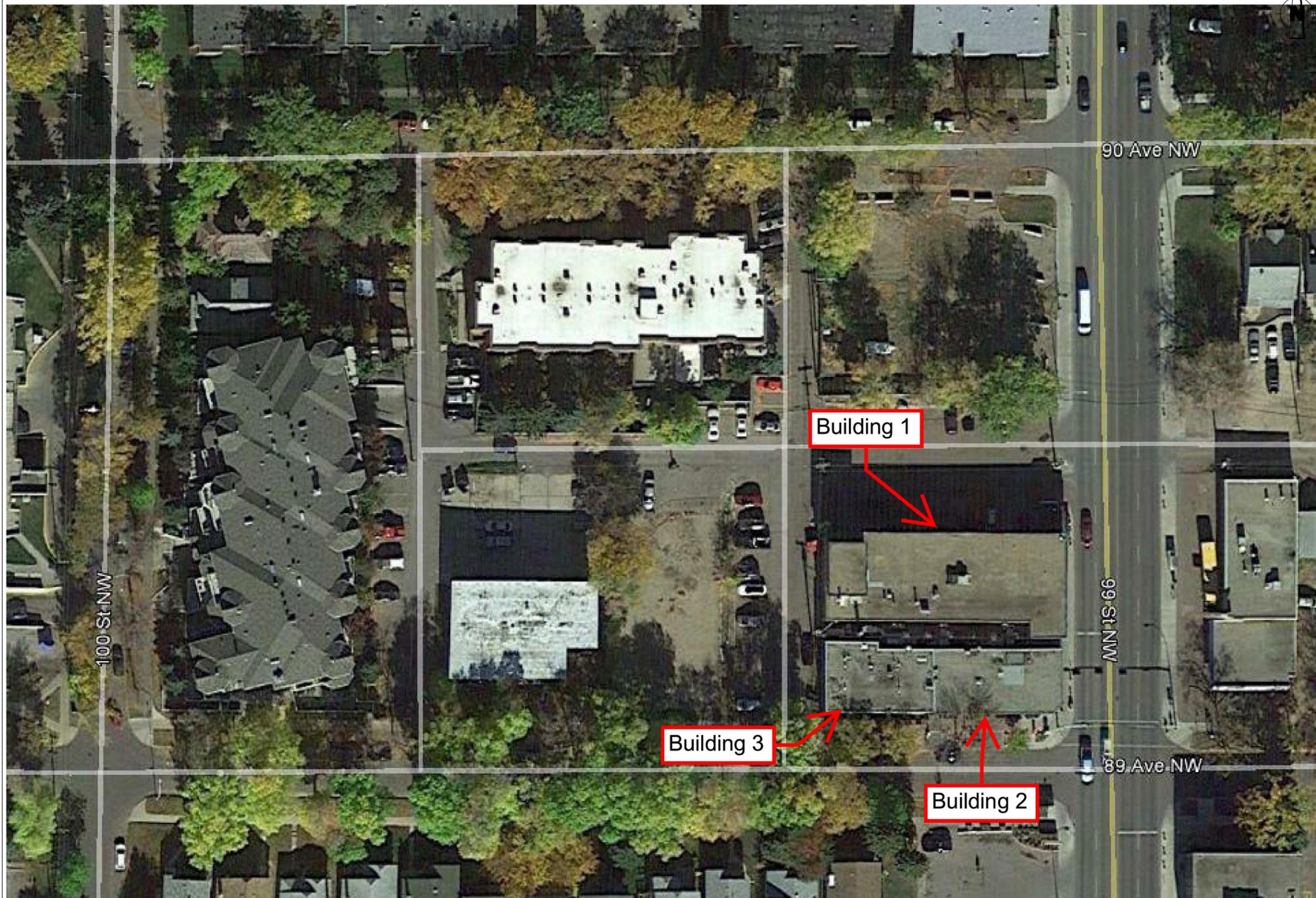
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ORIGINAL SCALE: <b>N.T.S.</b>		IF THIS BAR IS NOT 25mm LONG, ADJUST YOUR PLOTTING SCALE.  25mm
DESIGNED BY: <b>B.C.</b>	DRAWN BY: <b>C.A.</b>	
CHECKED BY:	APPROVED BY:	



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CLIENT:  
  
 CLIENT REF. #:  
 PROJECT:  
**SITE SERVICING DESIGN BRIEF FOR  
 99th STREET MIXED USE DEVELOPMENT**

TITLE:  
**LOCATION PLAN  
 FIGURE A**



PROJECT NO: <b>161-17798</b>		DATE: <b>FEBRUARY 2018</b>
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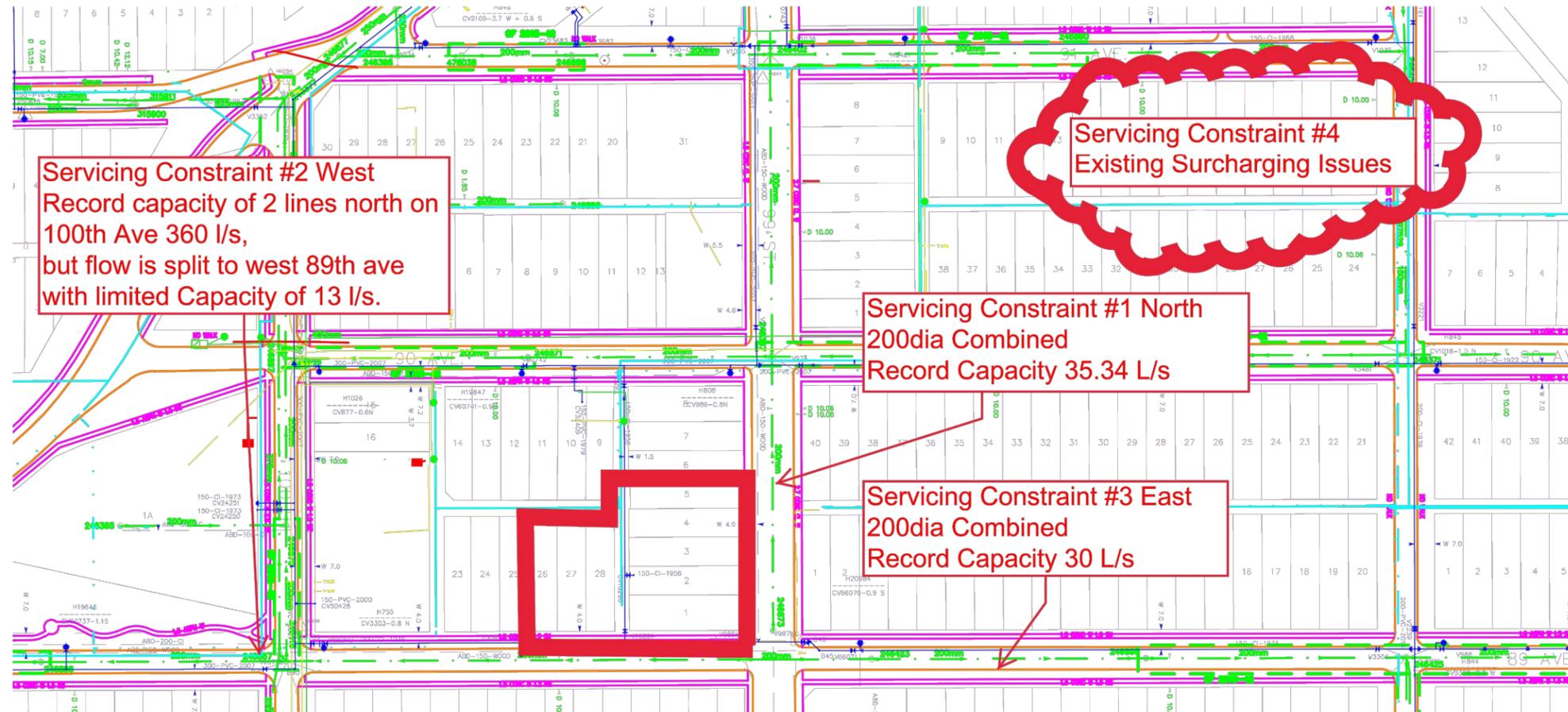
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PROJECT:

**SITE SERVICING DESIGN BRIEF FOR  
99th STREET MIXED USE DEVELOPMENT**

TITLE:  
  
**EXISTING BUILDING AND  
TOPOGRAPHIC CONFIGURATION  
FIGURE B**

N:\161-17798-00 99th St & 89th Ave Mix Use Project\6 Civil\Drawings\02 Figs-SKS\16117798-CF02.dwg Feb 08, 2018-1:11pm BY:(albardac)



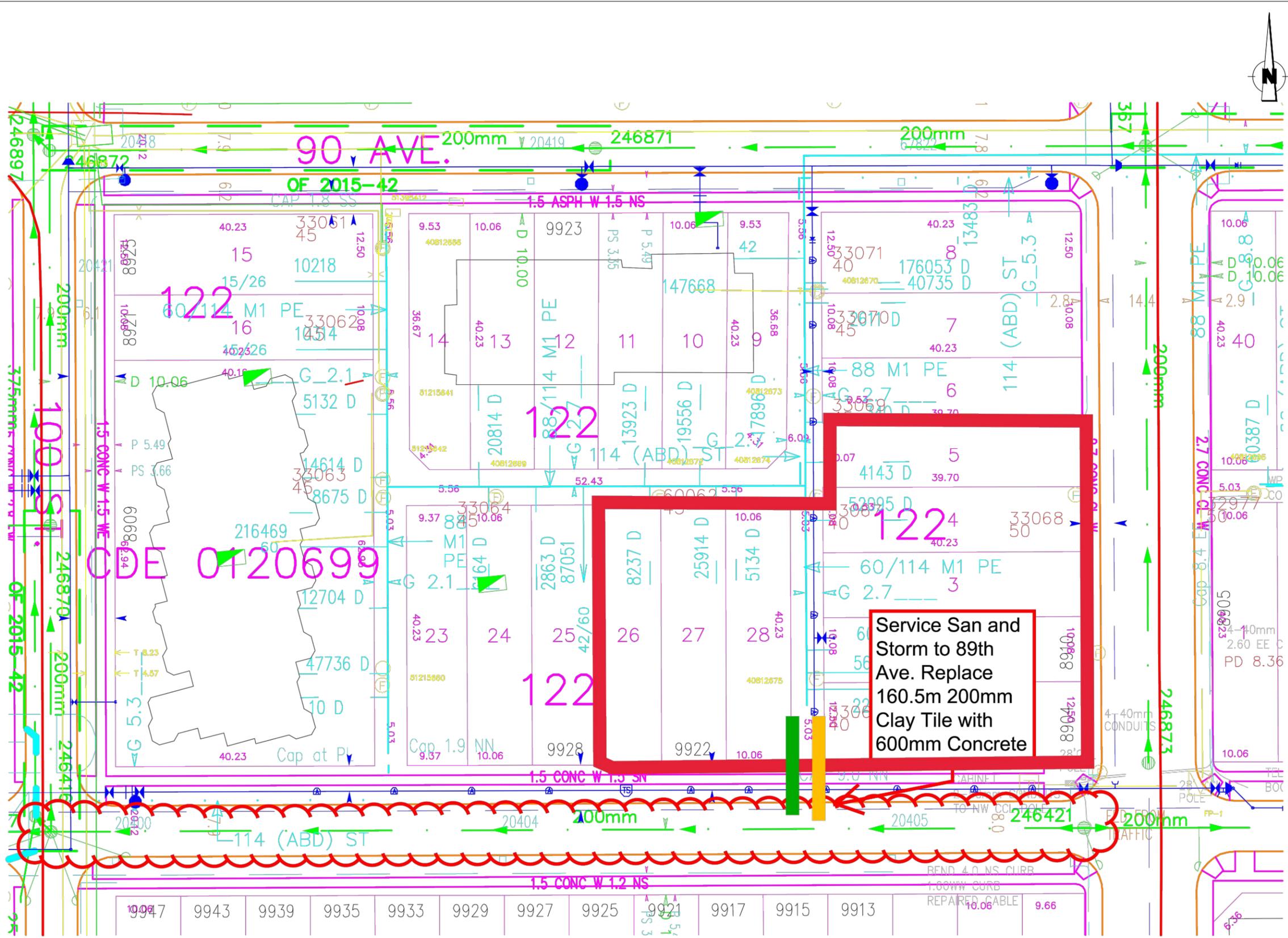
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PROJECT: <b>SITE SERVICING DESIGN BRIEF FOR 99th STREET MIXED USE DEVELOPMENT</b>

TITLE: <b>NEIGHBORHOOD CAPACITY CONSTRAINTS FIGURE C</b>
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CLIENT:  
  
CLIENT REF. #:  
PROJECT:

SITE SERVICING DESIGN BRIEF FOR  
99th STREET MIXED USE DEVELOPMENT

TITLE:  
  
PROPOSED SANITARY AND STORM  
SERVICING CONCEPT  
FIGURE D